PT-5-1

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND
DEPTH=39" TO TOP
RATE= 3 MPI

PT-5-2

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND
DEPTH=35" TO TOP
RATE= 2 MPI (UNABLE TO SATURATE)

PT D1

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO
DEPTH=42" TO TOP
RATE= 5 MPI

PT D2

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO
DEPTH= 63" TO TOP
RATE= <2 MPI

DH-5-1A

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND

NO REDOX. OBSERVED
WEEPING AT 104"
ESHGWT 52" (EST.)
REMOVE Ap AND Bw

SURFACE	EL.=387.7±
SL 10YR3/2	Ap 0"–10"
LS 10YR5/6	Bw 10"–37"
S 10YR5/1	C 37"–110"
BOTTOM OF HOLE	EL.=378.5±

DH-5-1B

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND

NO REDOX. OBSERVED
WEEPING AT 107"
ESHGWT 52" (EST.)
REMOVE Ap AND Bw

SURFACE	EL.=386.3±
SL 10YR3/4	Ap 0"–10"
S 10YR5/6	Bw 10"–36"
S 10YR6/1	C1 36"–48"
S 10YR5/1	C2 36"–122"
BOTTOM OF HOLE	EL.=376.1±

DH D1

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO

STANDING AT 102"
REFUSAL AT ±107"
ESHGWT 52"
REMOVE Ap, Bw AND C1

SURFACE	EL.=385.1±
SL 10YR3/2	Ap 0"–16"
SL 10YR4/6	Bw 16"–24"
SL 2.5Y5/3	C1 24"–38"
LS 2.5Y4/2	C2 38"–107"
BOTTOM OF HOLE	EL.=376.2±

DH D3

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO

STANDING AT 111"
ESHGWT 52"
REMOVE Ap AND Bw

SURFACE	EL.=385.3±
SL 10YR3/2	Ap 0"–8"
LS 10YR4/6	Bw 8"–18"
S 2.5Y3/2	C 18"–115"
BOTTOM OF HOLE	EL.=375.7±

DH-5-2A

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND

STANDING WATER AT 120"
REDOX AT 56" (10YR7/1)
REMOVE Ap AND Bw

SURFACE	EL.=385.7±
SL 10YR5/6	Ap 0"–10"
S 10YR6/4	Bw 10"–30"
S 2.5Y6/4	C1 30"–48"
S 2.5Y6/2	C2 48"–124"
BOTTOM OF HOLE	EL.=375.4±

DH-5-2B

DATE PERFORMED: 10/24/08
PERFORMED BY: KEVIN QUINN
WITNESS: SHELLEY HAMMOND

WEEPITNG AT 105"
REDOX AT 53" (2.5Y6/4)
REMOVE Ap AND Bw

SURFACE	EL.=384.6±
S 10YR3/2	Ap 0"–10"
S 10YR5/6	Bw 10"–36"
S 2.5Y6/1	C 36"–122"
BOTTOM OF HOLE	EL.=374.4±

DH D2

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO

STANDING AT 94"
ESHGWT 52"
REMOVE Ap, Bw AND C1

SURFACE	EL.=384.8±
SL 10YR3/2	Ap 0"–10"
SL 10YR4/6	Bw 10"–20"
LS 2.5Y6/4	C1 20"–38"
S 2.5Y3/3	C2 38"–98"
BOTTOM OF HOLE	EL.=376.6±

DH D4

DATE PERFORMED: 6/23/15
PERF. BY: CARL HULTGREN
WITNESS: BRIAN SZCZURKO

STANDING AT 111"
ESHGWT 52"
REMOVE Ap, Bw, C1 AND C2

SURFACE	EL.=384.8±
SL 10YR4/2	Ap 0"–8"
LS 10YR4/4	Bw 8"–20"
S 2.5Y5/2	C1 20"–41"
FINE S 2.5Y6/2	C2 41"–62"
S 2.5Y4/2	C3 62"–117"
BOTTOM OF HOLE	EL.=375.1±

PROPOSED ELEVATIONS

INV. AT FOUNDATION	389.03
INV. AT SEPTIC TANK INLET	388.23
INV. AT SEPTIC TANK OUTLET	387.88
INV. AT DBOX INLET	387.02
INV. AT DBOX OUTLET	386.85
EXIST. ELEV.	385.3
ESHGWT	381.0
BOTTOM FIELD	386.0
INV. AT END	386.50
INV. AT BEG.	386.70
BREAKOUT	387.20
MIN. FIN. GRADE	388.3+

RECORD ELEVATIONS

T.O.C.
INV. AT FND.
INV. AT SEPTIC TANK IN
INV. AT SEPTIC TANK OUT
INV. AT D-BOX IN
INV. AT D-BOX OUT
INV. AT BEGIN OF LINE #1
INV. AT END OF LINE #1
INV. AT BEGIN OF LINE #2
INV. AT END OF LINE #2
INV. AT BEGIN OF LINE #3
INV. AT END OF LINE #3
INV. AT BEGIN OF LINE #4
INV. AT END OF LINE #4

DESIGN CRITERIA:

TYPE OF BUILDING::	RESIDENTIAL
# OF BEDROOMS:	5
ESTIMATED DAILY FLOW:	550 GPD
GARBAGE GRINDER:	NO
PERCOLATION RATE:	<2 MPI AND 5 MPI
LTAR:	0.74 GPD/S.F.

DESIGN CALCULATIONS:

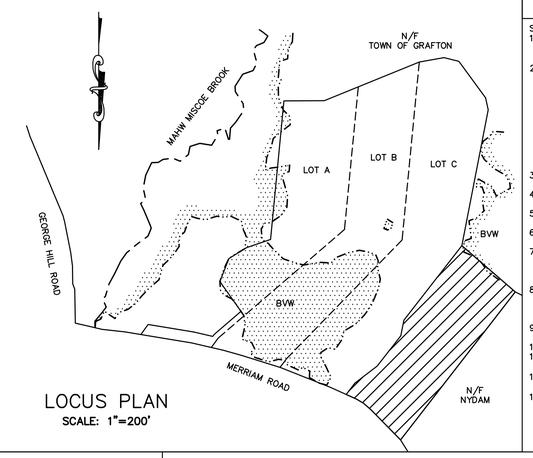
DESIGN PERC RATE:	<2 MPI
DESIGN DAILY FLOW:	550 GPD
LONG TERM ACCEPTANCE RATE (LTAR):	0.74 GPD/S.F.

LEACHING AREA REQUIRED BY TITLE V:
(550 GPD)/(0.74 GPD/S.F.) = 744 S.F.
LEACHING AREA DIMENSIONS PROVIDED: 23'W x 35'L
LEACHING CAPACITY PROVIDED UNDER TITLE V:
(23')(35')(0.74 GPD/S.F.) = 595 GPD
RESERVE AREA DIMENSIONS PROVIDED: 23'W x 35'L

SITE SPECIFIC NOTES:
1. PROPERTY INFORMATION:
STREET ADDRESS: 114 MERRIAM ROAD
ASSESSORS REF.: 110/059.0-0000-0024.
2. ZONING INFORMATION:
DISTRICT: AGRICULTURE/RESIDENTIAL

	REQUIRED:	PROVIDED:
AREA:	80,000 S.F.	99,056 S.F.
FRONTAGE/LOT WIDTH:	200 FT.	200.00 FT.
FRONT YARD:	30 FT.	70.0 FT
SIDE YARD:	15 FT.	49.0 FT.
REAR YARD:	15 FT.	390.2 FT.
BUILDING COVERAGE:	25%	2.7%
BUILDING HEIGHT:	35 FT.	<35 FT.

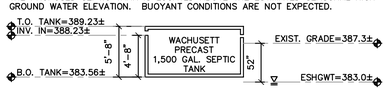
3. THE SITE DOES NOT LIE WITHIN THE GRAFTON WATER SUPPLY PROTECTION OVERLAY DISTRICT.
4. LOT D DOES NOT LIE WITHIN A FLOOD HAZARD ZONE DEPICTED ON NFP FIRM MAP 25027C0834E.
5. THE SITE DOES NOT FALL WITHIN A MAPPED PRIORITY HABITAT OF RARE SPECIES OR ESTIMATED HABITAT OF RARE WILDLIFE.
6. WETLAND RESOURCE AREAS WERE DELINEATED IN APRIL 2014 BY ECOTEC, INC.
7. ON AUGUST 22, 2014 THE GRAFTON CONSERVATION COMMISSION ISSUED AN ORDER OF RESOURCE AREA DELINEATION DETERMINING THAT THE BVW AND MAHWL OF MISCOE BROOK ARE ACCURATE.
8. ON AUGUST 25, 2014 THE GRAFTON CONSERVATION COMMISSION ISSUED A DETERMINATION OF APPLICABILITY UNDER THE GRAFTON WETLANDS PROTECTION BYLAW INDICATING THAT THE BOUNDARY DELINEATIONS ARE ACCURATE.
9. EXISTING CONDITIONS HAVE BEEN PROVIDED BY JARVIS LAND SURVEY, INC. UNLESS OTHERWISE NOTED.
10. ELEVATIONS ARE BASED ON NGVD88.
11. MISCOE BROOK WATERSHED IS DESIGNATED AS AN AREA OF CRITICAL ENVIRONMENTAL CONCERN (ACEC).
12. CONSTRUCTION VEHICLE TRAFFIC OVER THE PROPOSED INFILTRATION BASIN FLOOR(S) SHALL BE MINIMIZED.
13. WORK ASSOCIATED WITH THE HOUSE CONSTRUCTION ON LOT D RESULTS IN LESS THAN 40,000 S.F. OF DISRUPTION.



LOCUS PLAN

SCALE: 1"=200'

BUOYANCY CALCULATIONS:
ASSUME ELEVATIONS & TANK DIMENSIONS INDICATED.
ASSUME ESHGWT AT 52" BELOW EXISTING GRADE.
THE PROPOSED SEPTIC TANK IS LOCATED ABOVE THE ESTIMATED SEASONAL HIGH GROUND WATER ELEVATION. BUOYANT CONDITIONS ARE NOT EXPECTED.



SEPTIC TANK BUOYANCY INFORMATION (PROPOSED CONDITIONS)

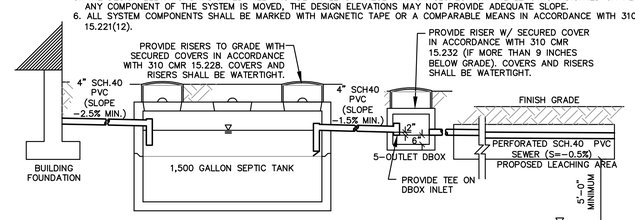
SCALE: 1/8"=1'-0"

NOTES:
1. SEE SCENIC ROAD PERMIT (SRP 2015-1) FOR REQUIREMENTS AT THE PROPOSED DRIVEWAY ACCESS.
2. SEE "LOT DEVELOPMENT PLANS GRAFTON, MASSACHUSETTS – 114 MERRIAM ROAD" (LATEST REVISION) FOR COMMON DRIVEWAY, DRAIN, ETC. CONSTRUCTION REQUIREMENTS.
3. SEE "STORMWATER REPORT" REVISED 5/11/15 FOR HYDROLOGIC CALCULATIONS INCLUDING LOTS A, B, C AND D.
4. THIS PLAN IS TO BE USED ONLY IN THE CONSTRUCTION OF THE PROPOSED LEACHING AREA ON LOT D.

LEGEND

XXX	EXIST. CONTOUR
XXX.XX	EXIST. SPOT GRADE
	EXIST. TREELINE
XXX	PROP. CONTOUR
XXX.XX	PROP. SPOT GRADE
	PROP. TREELINE
	DEEP HOLE
	PERCOLATION TEST

NOTES:
1. SEPTIC TANK AND DIST. BOX TO BE PLACED ON A LEVEL & STABLE BASE THAT HAS BEEN COMPACTED BY A VIBRATORY COMPACTOR OR EQUAL.
2. MIN. 5'–0" EXISTS BETWEEN BOTTOM OF LEACHING AREA AND MAX. G.W. ELEV.
3. A MINIMUM SLOPE OF –2.5% IS REQUIRED B/T THE HOUSE AND SEPTIC TANK.
4. A MINIMUM SLOPE OF –1.5% IS REQUIRED B/T THE SEPTIC TANK AND DISTRIBUTION BOX.
5. THE DESIGN ELEVATIONS INDICATED WILL PROVIDE AT LEAST THESE MINIMUM SLOPES FOR THE LENGTHS INDICATED ON PLAN. IF ANY COMPONENT OF THE SYSTEM IS MOVED, THE DESIGN ELEVATIONS MAY NOT PROVIDE ADEQUATE SLOPE.
6. ALL SYSTEM COMPONENTS SHALL BE MARKED WITH MAGNETIC TAPE OR A COMPARABLE MEANS IN ACCORDANCE WITH 310 CMR 15.221(12).

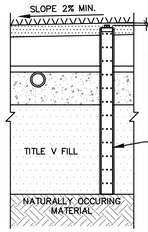


PROPOSED SEPTIC SYSTEM PROFILE

SCALE: NOT TO SCALE

SYSTEM NOTES:
1. PRIOR TO BEGINNING CONSTRUCTION, THE CONTRACTOR SHALL CONTACT THE ENGINEER TO VERIFY THAT THE CONSTRUCTION PLANS ARE THE MOST CURRENT REVISION.
2. THE INSTALLER SHALL SUBMIT TO THE ENGINEER A GRADATION ANALYSIS FOR THE FILL WITHIN THE SYSTEM TO DEMONSTRATE CONFORMANCE WITH 310 CMR 15.255(3) PRIOR TO PLACING THE FILL.
3. THE SYSTEM INSTALLATION SHALL BE INSPECTED BY THE LOCAL BOARD OF HEALTH AT THE MILESTONES AS DETERMINED BY THE LOCAL BOARD OF HEALTH.
4. THE INSTALLER SHALL NOTIFY QUINN ENGINEERING, INC. AT LEAST 24 HOURS IN ADVANCE OF REQUESTING INSPECTIONS. QUINN ENGINEERING, INC. SHALL CONDUCT INSPECTIONS AT THE FOLLOWING MILESTONES:
   A. EXCAVATION COMPLETE – PRIOR TO PLACING FILL
   B. INSTALLATION OF PIPING AND PEA STONE COMPLETE
   C. FINISH GRADING COMPLETE
   D. COMPLETE STABILIZATION
5. A SURVEYOR REGISTERED IN THE COMMONWEALTH OF MASSACHUSETTS SHALL STAKE OUT THE SYSTEM LOCATION AND SHALL PROVIDE AS-BUILT LOCATION AND ELEVATIONS TO QUINN ENGINEERING, INC. THE INSTALLER SHALL NOT COVER THE SYSTEM OR PIPING UNTIL QUINN ENGINEERING, INC. VERIFIES FROM THE AS-BUILT INFORMATION PROVIDED THAT THE SYSTEM LOCATIONS AND ELEVATIONS ARE SUITABLE.
6. ALL PORTIONS OF THE SPECIFIED SOIL HORIZONS SHALL BE REMOVED FROM WITHIN THE LIMITS OF THE LEACHING FACILITY AND FOR A DISTANCE OF FIVE FEET IN ALL DIRECTIONS THEREFROM (310 CMR 15.255 (5)). (SEE DEEP HOLE DATA FOR SOIL HORIZON INFORMATION.)
7. WHERE A SEWAGE DISPOSAL SYSTEM IS TO BE CONSTRUCTED IN FILL, THE FILL SHALL BE PLACED AND COMPACTED IN 12 INCH LIFTS OR ALLOWED TO SETTLE FOR A MINIMUM OF ONE YEAR. THE FILL MATERIAL MUST CONFORM WITH THE LOCAL BOARD OF HEALTH AND STATE ENVIRONMENTAL CODE TITLE 5 SECTION 15.255.
8. ALL SYSTEM PIPING SHALL BE MARKED WITH MAGNETIC MARKING TAPE IN ACCORDANCE WITH 310 CMR 15.221 (12).
9. ALL INTERIOR PLUMBING SHALL BE CONNECTED TO PROPOSED LEACHING FACILITY WITH THE EXCEPTION OF WATER SOFTENING OR CONDITIONING SYSTEMS, BACKWASH FROM FILTRATION SYSTEMS, OR FLOOR DRAINS.
10. SEVENTY TWO HOURS PRIOR TO COMMENCING ANY EXCAVATION, THE CONTRACTOR SHALL NOTIFY DIGSAFE AT (888) 344–7233. THE CONTRACTOR SHALL COORDINATE INSTALLATION OF NEW UTILITIES WITHIN THE VICINITY OF EXISTING UTILITIES (UNDERGROUND AND OVERHEAD) WITH THE APPROPRIATE UTILITY PROVIDER.
11. MACHINERY WHICH MAY CRUSH OR DISTURB THE PIPE SHALL NOT BE ALLOWED ON THE DISPOSAL AREA.
12. EXISTING UTILITY LOCATIONS ARE APPROXIMATE ARE TO BE FIELD VERIFIED. QUINN ENGINEERING, INC. DOES NOT WARRANT THAT ALL EXISTING UTILITIES HAVE BEEN INDICATED. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE EXISTING UTILITY LOCATIONS AND ENSURING THAT THE PROPOSED WORK DOES NOT CONFLICT WITH THE EXISTING UTILITIES (SHOWN OR NOT SHOWN).
13. ALL MODIFICATIONS TO THIS PLAN MUST BE PRE APPROVED IN WRITING BY THE DESIGN ENGINEER AND THE LOCAL BOARD OF HEALTH.
14. THE SYSTEM WAS NOT DESIGNED TO FACILITATE A GARBAGE DISPOSAL.
15. THE CONSTRUCTION OF PERMANENT STRUCTURES UPON THE DISPOSAL SYSTEM OR RESERVE AREA IS NOT ALLOWED.
16. ALL CONSTRUCTION METHODS AND MATERIALS SHALL CONFORM WITH THE LOCAL BOARD OF HEALTH AND THE STATE ENVIRONMENTAL CODE TITLE 5 (310 CMR 15.000).
17. SEWERS SHOULD BE LAID AT A MINIMUM OF 10 FEET, HORIZONTALLY, FROM ANY EXISTING OR PROPOSED WATER LINE. SHOULD LOCAL CONDITIONS PREVENT LATERAL SEPARATION OF 10 FEET TO A WATER LINE, THE WATER LINE SHOULD BE LAID IN A SEPARATE TRENCH AND THE ELEVATION OF THE CROWN OF THE SEWER PLACED AT LEAST 18 INCHES BELOW THE INVERT OF THE WATER LINE. WHENEVER SEWERS MUST CROSS UNDER WATER, THE SEWER SHOULD BE LAID AT SUCH AN ELEVATION THAT THE CROWN OF THE SEWER IS AT LEAST 18 INCHES BELOW THE INVERT OF THE WATER LINE. WHEN THE ELEVATION OF THE SEWER CANNOT BE VARIED TO MEET THIS REQUIREMENT, THE WATER LINE SHOULD BE RELOCATED TO PROVIDE THIS SEPARATION OR CONSTRUCTED WITH MECHANICAL-JOINT PIPE FOR A DISTANCE OF 10 FEET ON EACH SIDE OF THE SEWER. ONE FULL LENGTH OF WATER LINE SHOULD BE CENTERED OVER THE SEWER SO THAT BOTH JOINTS WILL BE AS FAR FROM THE SEWER AS POSSIBLE. WHEN IT IS IMPOSSIBLE TO OBTAIN HORIZONTAL OR VERTICAL SEPARATION AS STIPULATED ABOVE, BOTH SEWER AND WATER LINE SHOULD BE ENCASED IN CONCRETE FOR 10 FEET EITHER SIDE OF THE CROSSING.
18. THE SYSTEM OWNER SHALL HAVE A SEPTAGE HANDLER, LICENSED BY THE LOCAL BOARD OF HEALTH IN ACCORDANCE WITH G.L.C. 111 S. 31A AND 310 CMR 15.502, SERVICE THE EFFLUENT TEE FILTER REGULARLY, AT LEAST ONCE EVERY YEAR AND PUMP THE SEPTIC TANK IN ACCORDANCE WITH 310 CMR 15.000. THE SYSTEM OWNER SHALL REPORT IN WRITING TO THE LOCAL BOARD OF HEALTH WITHIN 30 DAYS OF THE DATE OF SERVICING EVERY TIME THE SEPTIC TANK IS SERVICED.
19. ALL COMPONENTS OF THE SYSTEM SHALL BE MAINTAINED IN ACCORDANCE WITH 310 CMR 15.351 OR OTHER APPLICABLE SECTION OF 310 CMR 15.
20. A SIGN SHALL BE PROVIDED OF DURABLE MATERIAL TO INDICATE TO THE PROPERTY OWNER THAT THE SEPTIC TANK IS EQUIPPED WITH AN EFFLUENT FILTER TEE. TIES TO THE TEE FROM PERMANENT MARKERS SHALL BE INDICATED ON THE SIGN. THE SIGN SHALL BE PLACED IN A CONSPICUOUS LOCATION AT THE PROPERTY OWNER'S DISCRETION.
21. THE GROUND IMMEDIATELY ADJACENT TO FOUNDATIONS SHALL BE SLOPED AWAY FROM THE BUILDING, IN ACCORDANCE WITH MA STATE BUILDING CODE (CURRENT ED.)
22. FILL REQUIRED BELOW ANY PROPOSED BUILDING FOUNDATION SHALL BE SPECIFIED IN ACCORDANCE WITH THE MA STATE BUILDING CODE (CURRENT ED.) AND SHALL BE SPECIFIED AS STRUCTURAL FILL BY A MASSACHUSETTS REGISTERED STRUCTURAL ENGINEER. QUINN ENGINEERING, INC. DOES NOT SPECIFY SOIL TO BE USED BELOW BUILDING FOOTINGS OR AS STRUCTURAL FILL.
23. CONTRACTOR SHALL INSTALL A FOUNDATION DRAIN AND/OR PUMP AS NECESSARY TO REMOVE GROUNDWATER & WATER PROOF THE PROPOSED FOUNDATION IN ACCORDANCE IN ACCORDANCE WITH THE MA STATE BUILDING CODE (CURRENT EDITION).
24. ALL KNOWN WELLS OR LEACHING AREAS WITHIN 150' OF THE PROPOSED LEACHING AND RESERVE AREA HAVE BEEN INDICATED.
25. ALL KNOWN CATCH BASINS, DRYWELLS, CULVERTS, DETENTION BASINS, OR STORMWATER DRAINS WITHIN 50' OF THE EXISTING WELL HAVE BEEN INDICATED.
26. THERE ARE NO PUBLIC WATER SUPPLY WELLS WITHIN 200' OF THE PROPOSED LEACHING AREA.
27. ALL KNOWN STORMWATER SYSTEMS AND DISCHARGES LOCATED ON SITE HAVE BEEN INDICATED.
28. ALL KNOWN EXISTING LAND USES WITHIN 200' OF THE EXISTING WELL ARE EITHER RESIDENTIAL OR OPEN SPACE.
29. ALL KNOWN EXISTING AND PROPOSED SUBSURFACE FUEL STORAGE TANKS HAVE BEEN INDICATED.
30. THERE ARE NO KNOWN WASTE SITES WITHIN 500'–1000' OF THE SITE.
31. THERE ARE NO KNOWN CURRENTLY USED BARNYARDS OR CORRALS WITHIN 100' OF THE EXISTING WELL.
32. THE SITE WAS PREVIOUSLY USED AS A FIELD.
33. THERE ARE NO KNOWN OTHER WATER SUPPLY LINES WITHIN 10' OF THE EXISTING WELL.
34. THERE ARE NO KNOWN PETROLEUM STORAGE TANKS, FUEL LINES, OR PUMPS WITHIN 100' OF THE EXISTING WELL.
35. THERE ARE NO KNOWN STABLES, BARNS, FEEDLOTS, MANURE STORAGE AREAS WITHIN 100' OF THE EXISTING WELL.
36. KNOWN AND PROPOSED GAS LINES IN THE VICINITY OF THE EXISTING WELL HAVE BEEN SHOWN.
37. KNOWN TRANSMISSION LINES WITHIN THE VICINITY OF THE EXISTING WELL HAVE BEEN SHOWN.
38. THERE ARE NO KNOWN CHEMICAL MIXING AREAS WITH CONTAINMENT STRUCTURE WITHIN 200' OF THE EXISTING WELL.
39. THERE ARE NO KNOWN SALT STORAGE AREAS WITHIN 100' OF THE EXISTING WELL.
40. FILTER FABRIC SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

PHYSICAL:		
GRAB TENSILE STRENGTH:	90 LBS	ASTM D–4632
PUNCTURE:	65 LBS	ASTM D–4833
HYDRAULIC:		
APPARENT OPENING SIZE:	70 US SIEVE	ASTM D–4751
PERMITIVITY:	2.5 SEC.–1	ASTM D–4491
FLOW RATE:	175 GAL./MIN./SQ.FT.	ASTM D–4491

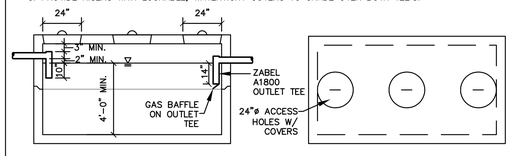


NOTE:
1. PROVIDE ONE INSPECTION PORT WITHIN THE LEACHING AREA.
2. THE OBSERVATION PORT SHALL CONFORM TO 310 CMR 15.240(13).

PROPOSED OBSERVATION PORT SECTION

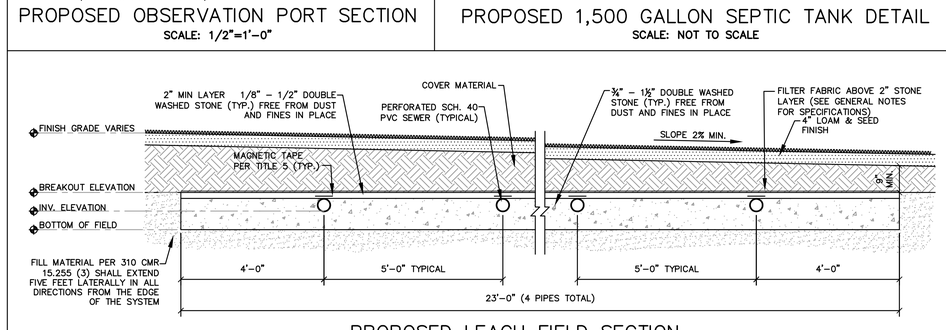
SCALE: 1/2"=1'-0"

NOTE:
1. TANK CONSTRUCTION SHALL CONFORM TO 310 CMR 15.000.
2. TANK SHALL BE CAPABLE OF CARRYING H10 WHEEL LOAD WITH THE PROP. SOIL OVERBURDEN.
3. PROVIDE RISERS WITH LOCKABLE, WATERTIGHT COVERS TO GRADE OVER BOTH TEE'S.



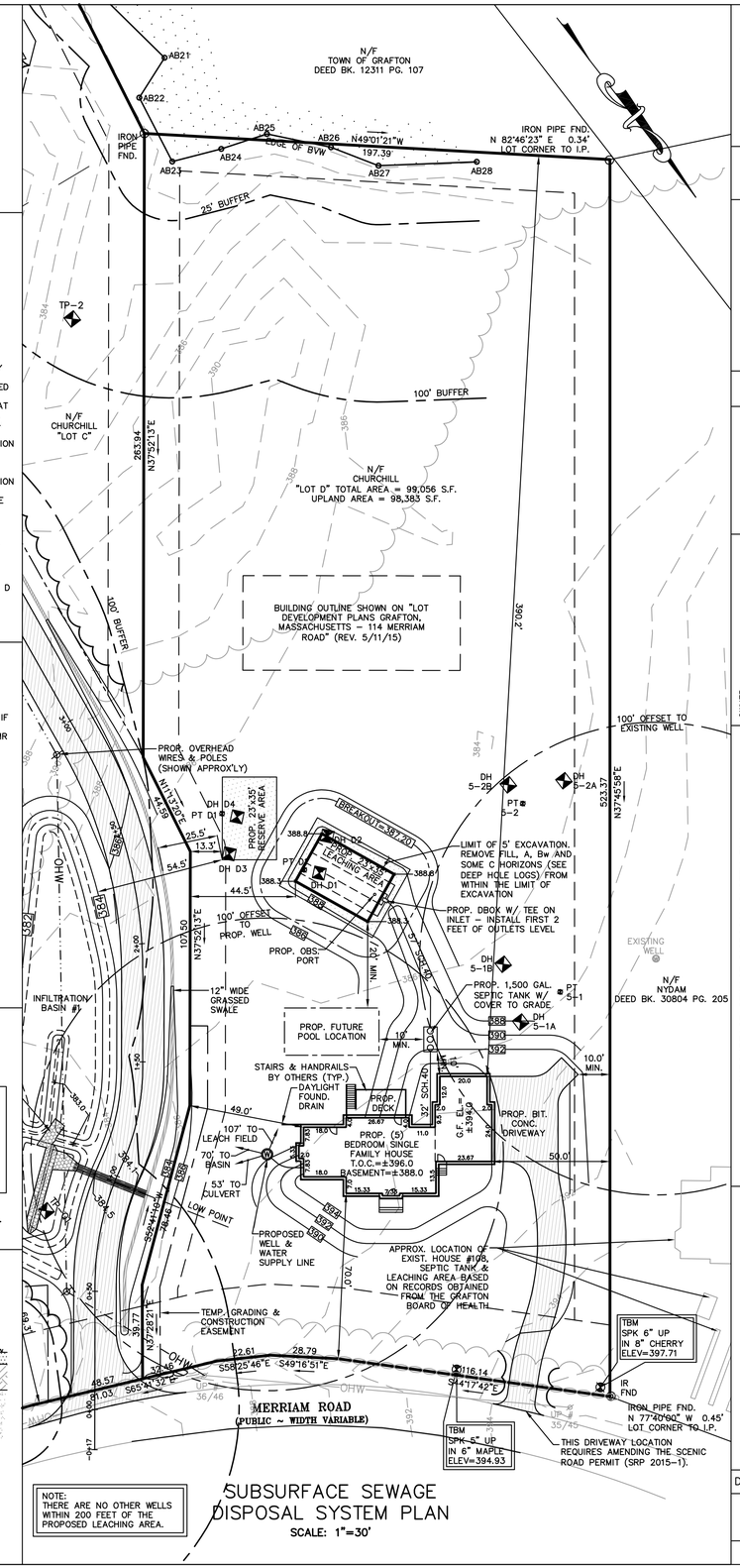
PROPOSED 1,500 GALLON SEPTIC TANK DETAIL

SCALE: NOT TO SCALE



PROPOSED LEACH FIELD SECTION

SCALE: NOT TO SCALE



NOTE:
THERE ARE NO OTHER WELLS WITHIN 200 FEET OF THE PROPOSED LEACHING AREA.

SUBSURFACE SEWAGE DISPOSAL SYSTEM PLAN

SCALE: 1"=30'

DIG SAFE
DIAL-811

NO.	REVISION	DATE

KEVIN J. QUINN CIVIL #33922

OWNER:
FREDERIC & MARGOT CHURCHILL
243 PLEASANT STREET
S. YARMOUTH MA 02664

APPLICANT:
FREDERIC CHURCHILL
243 PLEASANT STREET
S. YARMOUTH MA 02664

SUBSURFACE SEWAGE DISPOSAL SYSTEM PLAN
IN GRAFTON, MASSACHUSETTS

LOT D – 114 MERRIAM ROAD

30 0 30 60 120
SCALE: 1"=30'

QUINN ENGINEERING, INC.
P.O. Box 107
Paxton, Massachusetts 01612
(508)753-7999 Fax:(508)795-0939

DATE: AUGUST 17, 2016

SUBSURFACE SEWAGE DISPOSAL SYSTEM PLAN

SHEET 1 OF 1

116BC02